The durability of normal strength concrete: an experimental study

V. Patel & N. Shah Civil Engineering Department, Charotar University of Science and Technology, India

Abstract

The present study is carried out for evaluating the effect of aggressive environments like sulphate and chloride exposure on normal strength concrete. This paper presents results of experiments like the compression test (150mm×150mm×150mm size cubes), rebound hammer test and ultrasonic pulse velocity test (150mm×150mm×150mm size cubes), split tensile test (100mm diameter×200mm height cylinders), rapid chloride penetration test (100mm diameter, 50mm thickness specimen), sulphate and chloride attack test (150mm×150mm×150mm size cubes) performed on M₃₀ grade concrete. The results clearly indicated that normal strength concrete cannot contribute significantly against sulphate and chloride attack.

Keywords: normal strength concrete, aggressive environment, sulphate and chloride attack, rapid chloride penetration.

1 Introduction

Normal strength concrete is a widely used construction material with a compressive strength of 20MPa to 40MPa. The durability of normal strength concrete is very important for a particular application and environment to which it will be exposed. Premature deterioration of concrete long before its service life has become a critical issue [1]. Much research has been done to increase the strength of concrete by adding different admixtures and checking the behaviour of concrete under different environments like industrial effluents [2]. Permeability depends upon on the extent of hydration of cementitious material. The curing condition and age of concrete largely determines the ease with which chloride ions can move into a concrete [3]. Concrete deterioration occurs when



it is exposed to weather, water or chemicals over a long period of time. The durability of concrete is of vital importance regarding the life cycle cost of a structure [4]. The life cycle cost includes initial cost of materials and labour and cost of maintenance and repair. The selection of proper materials and good quality control are essential for durability of concrete. The chloride dissolved in water increase the rate of leaching and thus increases the porosity of concrete and strength and stiffness decreases. Sodium and magnesium sulphates are in increasing order harmful to concrete as they react with hydrated cement paste which decreases the strength [5]. When RCC structures are exposed to harsh environments, deterioration of concrete occurs due to chloride and sulphate attack. Therefore, the degree of exposure of the environment condition to which concrete is subjected over its entire life is also very important [6]. This research was performed to generate data on durability characteristic of normal strength concrete so later on value added high performance concrete can be developed which can overcome all durability related deficiencies of normal strength concrete

2 Experimental study

2.1 Materials used

2.1.1 Cement

Ordinary Portland cement Sanghi-53 grade confirming IS 12269: 1987 [7] was used in the present study with properties listed in Table 1.

Sr. No.	Properties	Results
1	Loss of ignition	1.56
2	% SiO ₂	19.70
3	% CaO	63.44
4	Specific gravity	3.15
5	% Normal consistency	29.5
6	Specific surface (m ² /kg)	306
7	Initial setting time (minutes)	145
8	Final setting time (minutes)	185
9	Compressive strength (MPa) 3days	37.5
	7days	48.5
	28days	63.0

 Table 1:
 Properties of OPC 53 grade Sanghi cement.



2.1.2 Coarse aggregate

Crushed angular aggregates confirming IS 383: 1970 [8] with maximum size of 20mm was used having a bulk density of 1600 kg/m³. The specific gravity was found to be 2.81.

2.1.3 Fine aggregate

River sand from local sources confirming IS 383: 1970 [8] was used as fine aggregate. The fineness modulus, specific gravity and water absorption were found to be 2.64, 2.82 and 1.87% respectively.

2.1.4 Water

Fresh potable water free from acid and organic substances was used for mixing and curing the concrete.

2.2 Mix proportion

The mix proportion shown in Table 2 was made for a concrete with slump 30–50mm and M_{30} grade as per mix design method specified by ACI committee 211.1-91 [14].

W/C ratio	Cement (kg)	Fine aggregate (kg)	Coarse aggregate (kg)	Water (liter)
0.45	411	880	992	185

Table 2: Mix proportion of M₃₀ grade concrete.

2.3 Casting and curing

Mixing of ingredients was done according to specifications given in IS 516: 1959 [9] by machine mixing. The concrete was filled into the moulds in layers approximately 5cm deep and compacted by vibrator. The specimens were removed from the moulds after 24 hours and were kept submerged in curing tank. After curing for a period of 7 and 28 days, specimens were taken out and dried before testing.

2.4 Testing

Specimens were tested to ascertain properties at 7 and 28 days by performing the following tests.

2.4.1 Compression test

A compression test on cubes of size 150mm×150mm×150mm was conducted on the compression testing machine (Aimil, 2000kN capacity). The load on cube was applied at a rate 5.2kN/s up to the failure of specimen. The average compressive strength of three cubes was taken after 7 and 28 days curing.



Date of casting	Dime	nsions ((mm)	Test res	ults at 7 days	Test results at 28 days		
0	Length	Width	Thick-	Failure	Compressive	Failure	Compressive	
	_		-ness	load	stress	load	stress	
				(kN)	(Mpa)	(kN)	(Mpa)	
25/01/13	150	150	150	514.6	22.87	943	41.91	

Table 3:Compression test results on cubes.

2.4.2 Rebound hammer test

The rebound hammer test was performed on cubes of size 150mm×150mm×150mm×150mm by pressing a plunger of a rebound hammer against the surface of concrete vertically downward as specified in IS 13311 (part 2): 1992 [10]. The rebound was read off along a graduated scale and was designated as the rebound number.

Table 4:Rebound hammer test results.

Date of casting	Dime	ensions (1	nm)	Test res	ults at 7 days	Test results at 28 days	
custing	Length	Width	Thick-	Rebound	Compressive	Rebound	Compressive
	-		-ness	number	strength (MPa)	number	strength (MPa)
25/01/13	150	150	150	22.33	18.4	31.66	33.66

2.4.3 Ultrasonic pulse velocity test

Mix proportion used in making, compacting and curing of concrete is very important as they affect the density and modulus of elasticity of concrete. The ultrasonic pulse velocity of concrete is related to these. Concrete quality was checked as per specifications given in IS 13311 (part 1): 1992 [11]. The ultrasonic pulse was produced by transducer which was held in contact with one surface of the concrete member. After travelling a known path length in the concrete, the pulse of vibration was converted into an electrical signal by the second transducer held in contact with the other surface of the concrete member and the transit time of the pulse was measured.

Date of casting	Di	mensions (mm)	Test results at 28 days			
	easting	Length	Width	Thickness	Transit time (µsec)	Pulse velocity (km/s)	Concrete quality grading
	25/01/13	150	150	150	34.73	4.32	Excellent

Table 5:Ultrasonic pulse velocity test results.



2.4.4 Split tensile test

Split tensile test on cylinders of size 100mm diameter and 200mm height was conducted on the compressive testing machine (Aimil, 2000kN) as per specifications given in IS 5816:1999 [12]. The load was applied at a rate of 1.8kN/s up to failure of specimen. The average split tensile strength of 3 cylinders was taken after 7 and 28 days.

Date of	Dimensions (mm)LengthDiameter		Test resul	ts at 7 days	Test results at 28 days		
casting			Failure	Tensile	Failure	Tensile	
			load (kN)	strength (MPa)	load (kN)	strength (MPa)	
09/02/13	200	100	65.8	2.10	94.1	2.99	

Table 6:	Split tensile strength test results.
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2.4.5 Sulphate and chloride exposure test

5% Na₂SO₄ and 5% NaCl by volume solutions were prepared and three cubes of size 150mm×150mm×150mm were immersed in each solution after 28 days normal curing for 28 days after taking initial weights. The cubes were weighted and tested on compression testing machine after 28 days.



Figure 1: Sulphate exposure of cubes.

Date of casting	Dimen	sions (1	nm)		Test result	ults at 28 days			
custing	L W T			Weigl	ht (kg)	% loss in wt.	Failure load	Compr. str.	
				Before immersion	After immersion	iii we.	(kN)	(MPa)	
25/01/13	150	150	150	8.337	8.373	+0.431	759.67	33.76	



Date of casting	Dime	nsions (mm)		Test rest	sults at 28 days			
custing	L	W	Т	Weight (kg)		% loss in	Failure load (kN)	Compr. Str.	
				Before immersion	After immersion	weight	iouu (ki i)	(MPa)	
25/01/13	150	150	150	8.327	8.363	+0.432	785	34.89	

Table 8: Immersion in NaCl solution test results.

2.4.6 Rapid chloride penetration test

As per ASTM C1202-12 [13], the amount of electric current passed through 50mm thick slices of 100mm diameter cylinder was monitored during 6h period as shown in Fig. 2. A potential difference of 60V DC was maintained across the ends of the specimen, one of which was immersed in a sodium chloride solution, the other in a sodium hydroxide solution. The total charged passed in coulombs was found.



Figure 2: Rapid chloride penetration test.

3 Results and discussions

For fresh concrete slump obtained was 32mm and compaction factor was 0.848. From Fig. 3, it can be seen that compressive strength of cubes at 7 days is 22.87MPa and at 28 days 41.91MPa. The increase in strength is about 83.25%. From the rebound hammer test results as shown in fig. 4, the compressive strength of cube at 7 and 28 days is coming 18.4MPa and 33.66MPa corresponding to 18.4 and 31.66 rebound Number. The results of ultrasonic pulse



velocity shown in Table 5 indicate that quality of concrete was excellent as ultrasonic pulse velocity is 4.32km/s. Fig. 4 shows that the split tensile strength of cylinders at 7 and 28 days are 2.10MPa and 2.99MPa respectively. The increase in strength is about 42.38%. Table 7 indicates that there is an increase in weight of cubes when immersed in Na₂SO₄ solution for 28 days by 0.368kg and compressive strength is 33.76MPa. Therefore there is an increase in weight by 0.431% and decrease in strength by 19.45%. Table 8 indicates that there is an increase in weight of cubes when immersed in NaCl solution for 28 days by 0.360kg and compressive strength is 34.89MPa. Therefore there is an increase in weight by 0.432% and decrease in strength by 18.75%. Table 9 indicates that there is increase in chloride ion permeability by 11% at 56 days.

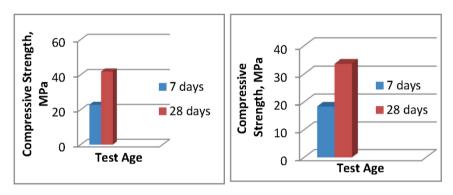
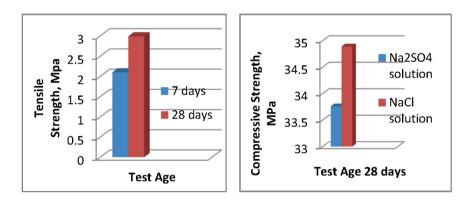
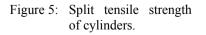
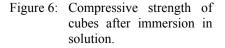


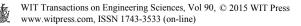
Figure 3: Compressive strength of cubes.

Figure 4: Compressive strength from rebound number.









Date of casting	Dimensions (mm)		Test results	at 28 days	Test results at 56 days		
	L	D	• •	Chloride ion permeability	Charge passed (coulomb)	Chloride ion permeability	
09/02/13	50	100	2241	Moderate	2520	Moderate	

 Table 9:
 Rapid chloride penetration test results.

4 Conclusions

The following are the conclusions from the present study.

- Normal strength concrete is excellent as far as normal exposure is considered as it has shown very good compressive strength at 7 and 28 days. 7 days' strength was more than 2/3 of 28 days strength.
- Non destructive tests also confirmed good quality of concrete as pulse velocity and rebound hammer number were 4.32km/s and 31.66 respectively.
- Split tensile strength was around 1/10 of expected compressive strength at 7 and 28 days which indicate good casting quality.
- The normal strength concrete casted shows marginal increase in weight when exposed to Na_2SO_4 and NaCl solution for 28 days but Compressive Strength reduces drastically by 19.45% and 18.75% respectively.
- Na₂SO₄ decompose the C-S-H and produce ettringite which has no binding properties and therefore reduce the strength. The failure mode of Na₂SO₄ exposed concrete is different than conventional concrete. In Na₂SO₄ exposed cubes, cracks propagate rapidly through the cubes on increasing load and get crushed. NaCl exposed concrete fails almost in similar way as conventional concrete.
- Chloride ion permeability increased by 11% after 56 days shows that at higher ages normal strength concrete remains porous even if hydration is completed.
- From the research it was proposed that, to improve mechanical and durability related properties of normal strength concrete, mineral and chemical admixtures have to be utilized in making concrete.

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