

Application of the shakedown analysis in the elastic: plastic assessment of cracked plates

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Abstract

In this work, a new method in the elastic-plastic calculation of cracked plates, based on shakedown analysis, is presented. This method presents an interesting alternative to classical methods in the structure design, especially when the loading is variable. It permits, via a simple elastic calculation and a mathematical optimisation, the determination of a load domain situated between the elastic and the limit load domains, within which the loading can evolve arbitrarily, while insuring the reliability of the cracked structure versus the plastic ruin and the unstable crack propagation. Also, by taking into account the microstructure of the material and the residual stresses, this method allows the determination of a stress intensity factor corresponding to the shakedown state. This factor is comparable to the fatigue threshold and can be used in the crack admissibility criteria.

Keywords: shakedown, crack, notch, finite elements analysis, stress intensity factor, fatigue threshold.

1 Introduction

The structures used in the aeronautics, nuclear plants and naval yards are generally plane forms, originally, constituted of ductile material plates (metals and alloys) that could support irreversible plastic deformations before to break, and that even when they contain manufacture defects or cracks.

Cracks constitute the major problem of designers, because they can falsify completely the prediction of the conceived structure behavior, by accelerating their ruin through brutal propagation of these cracks.

When a crack is detected in a system, the designer undertakes an analysis, where he will have to explore the brutal rupture risk, the plastic ruin risk and the foreseeable evolution of the fissure.



The most often, because of the geometrical singularity fathered by the crack, one uses the fracture mechanics, that has replaced the classical design methods.

Nevertheless, one knows that the fracture mechanics alone cannot insure that a crack is admissible; it attends only what happens at the vicinity of the crack tip (front), where the strong constraints and deformations can cause plastic yield of the material and therefore the propagation of the crack.

Criteria of defect admissibility of the fracture mechanics are naturally local, attached to event being able to happen at the crack tip. One knows also that even flawless, a structure cannot support unlimited loads, it finishes by bankrupting.

To take into account these two modes of ruin and to anticipate the stability and the security of the cracked structure, Belouchrani and Weichert [1][2], have proposed a new approach by the application of the shakedown theory, that finds its preferential application framework when the loading is variable. This new approach consists of an extension of the static shakedown theorem to the cracked structures.

The goal of this work is to present this new prediction method of the cracked plate security and reliability, by using the finite elements method and the non-linear mathematical optimization.

The crack is assimilated to a sharp notch according to Neubers material block concept [3].

2 Formulation of the shakedown theorem for a cracked body

It has been suggested [1,2], that a cracked body shakes down with respect to a given loading history, if a time-independent state of residual stress $\rho^\circ(x)$ exists, such that for all times $t > 0$:

$$\rho_{ij,j}^\circ = 0 \quad \text{in } \Omega \quad (1)$$

$$n_j \rho_{ij}^\circ = 0 \quad \text{on } \Gamma_\sigma \quad (2)$$

$$F(\sigma_{ij}^c(t) + \rho_{ij}^\circ, \sigma_y) < 0 \quad \text{in } \Omega \quad (3)$$

With a supplementary condition imposed on the admissible length of a typical micro-crack in the material

$$a_{lim} < a_c \quad (4)$$

Here, F is the plastic yield surface assumed of Von-Mises type, convex by definition, σ_y is the yield stress and a_{lim} is the largest admissible crack length determined by means of shakedown analysis. In inequality (3), $\sigma^c(x, t)$ is the time-dependent stress state for a purely elastic comparison problem, differing from the original problem only by the fact that the material reacts purely elastically with the same elastic moduli as for the elastic part of the material law in the original problem. For the Lemaitre-Chaboche model [4] adopted for ductile fracture, a_{lim} is given by [1,2]

$$a_{lim} = a_0 + \left[\frac{m+1}{m} \frac{1}{K} \frac{\alpha}{\alpha-1} \int_{\Omega} \frac{1}{2} (\rho_{ij}^\circ L_{ijkl} \rho_{kl}^\circ) d\Omega \right]^{m/(m+1)} \quad (5)$$



Here, a_0 is the initial crack length, \mathbf{L} is the positive definite and time-independent tensor of elastic moduli, m and K are material constants characterizing the R-curve parameters and α is the shakedown safety factor.

As mentioned before, the problem of stress singularity of the elastic stress field deserves special attention for the application of the shakedown theorem to cracked structures. In this case, no time-independent field of residual stresses $\rho^\circ(x)$ satisfying inequality (3) can be found and classical shakedown theory does not deliver comprehensive results, even for loads for which limit states physically exist. We bypass this problem by assimilating the crack tip to a notch, following the concept of material block introduced by [3] and used in the same spirit as in the present work by [5].

So, following [6], the stress distribution in the neighborhood of the root of the notch given by

$$r_f = r + n\varepsilon \tag{6}$$

with, according to figure 1, r_f as the effective notch root radius, ε as the length of the Neuber material block (assumed to be a material constant), and n as the factor depending on the loading mode. The factor n is equal to 2 in mode I. Following this concept, the effective notch radius is equal to the original notch radius augmented of n times the dimension of the Neuber material block.

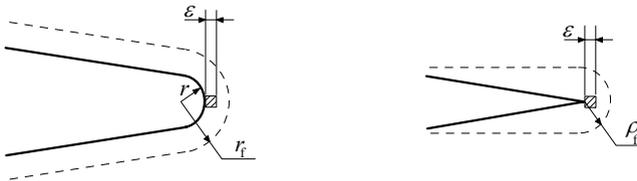


Figure 1: Modified notch and crack.

In the case of a sharp crack, the radius at the root of the effective crack, denoted ρ_f , can be obtained just by putting $r = 0$ in Eqn. (6)

$$\rho_f = n\varepsilon \tag{7}$$

Eqn. (7) indicates implicitly that the crack can be treated as a notch with ρ_f radius. Physically, Neuber's material block may be explained as being the sum of the minimum number of individual microscopic material particles (such as grains in polycrystalline metals). The properties of which may differ from each other, but in average they should have the property of the macroscopic material. In [5], ρ_f is put to be about ten times the size of a grain, for the mode I loading. Following this suggestion we write

$$\varepsilon \approx 5\xi \tag{8}$$

3 Shakedown safety factor

We consider an elastic-plastic plate, subjected to uniaxial loads $P(t)$. The values of $P(t)$ vary arbitrarily with time t , but remain between prescribed loads P_{\min}



and P_{\max} . One then looks for the maximum value of the load factor α , such that the plate will shake down under the load $\alpha P(t)$. This load factor will be called the shakedown load factor α_{SD} and can be determined as solution of the following optimization problem

$$\alpha_{SD} = \max_{\alpha, \rho^{\circ}} \alpha \quad (9)$$

with the subsidiary conditions

$$\rho_{ij,j}^{\circ} = 0 \quad \text{in } \Omega \quad (10)$$

$$n_j \rho_{ij}^{\circ} = 0 \quad \text{on } \Gamma_{\sigma} \quad (11)$$

$$F(\alpha \sigma^c(P) + \rho^{\circ}, \sigma_y) < 0 \quad \text{in } \Omega \quad \forall P \in [P_{\min}, P_{\max}] \quad (12)$$

$$a_{\text{lim}} = a_0 + \left[\frac{m+1}{m} \frac{1}{K} \frac{\alpha}{\alpha-1} \int_{\Omega} \frac{1}{2} (\rho_{ij}^{\circ} L_{ijkl} \rho_{kl}^{\circ}) d\Omega \right]^{m/(m+1)} < a_c \quad (13)$$

4 Fatigue threshold and influence of the microstructure

In what follows, the classical concept of existence of a threshold K_S for the stress intensity factor K_I in opening mode I is used.

$$K_I = \sigma \sqrt{\pi a} \quad (14)$$

Therefore, it is assumed that the micro-crack propagation does not occur if

$$K_I < K_S \quad (15)$$

and the material state is safe against failure by fatigue. Among other parameters, the fatigue threshold is influenced by the microstructure of the material. Generally, it is admitted that K_S increases with the size of the grain as has been observed on ferritic steels by [7–9]. It has been suggested that the dependence between K_S and $\xi^{1/2}$ (ξ is the diameter of the grain) are related by a linear function [7–9]

$$K_S = a_1 + b_1 \xi^{1/2} \quad (16)$$

where the dimensions of K_S and ξ are, respectively, $[\text{MPa} \cdot \text{m}^{1/2}]$ and $[\text{m}]$. Here, a_1 and b_1 are material constants.

5 Shakedown stress intensity factor K_{SD}

With the shakedown load factor α_{SD} computed for a cracked plate loaded in mode I, we will compute the stress intensity factor (Eqn. (14)) corresponding to the shakedown state by [10]

$$K_{SD} = \alpha_{SD} P \sqrt{\pi a_{\text{lim}}} \quad (17)$$



6 Numerical examples

Here, the finite element method is used combined with mathematical programming method to maximize the load factor under the subsidiary conditions (10)–(13) such that the yield criterion F is not violated in any point of the plate and that the admissible crack length a_{lim} remains inferior to its critical value a_c . The resolution of this mathematical programming problem is performed using the code LPNLP [11] that is based on an augmented Lagrangian method and the BFGS-algorithm. For the solution one needs:

- the solution of the purely elastic comparison problem in the sense of shakedown analysis,
- the construction of a time-independent residual stress field.

To this end, we use the finite element force method based on the principle of minimum complementary energy [12]. This approach uses stress functions for the construction of the complementary energy function and represents an algebraic dual to the finite element displacement method. This method has been used by [13] for the study of limit and shakedown analysis of two-dimensional structures.

6.1 Shakedown loads domain:

We consider a rectangular cracked plate, subjected to uniform loading (fig.2):

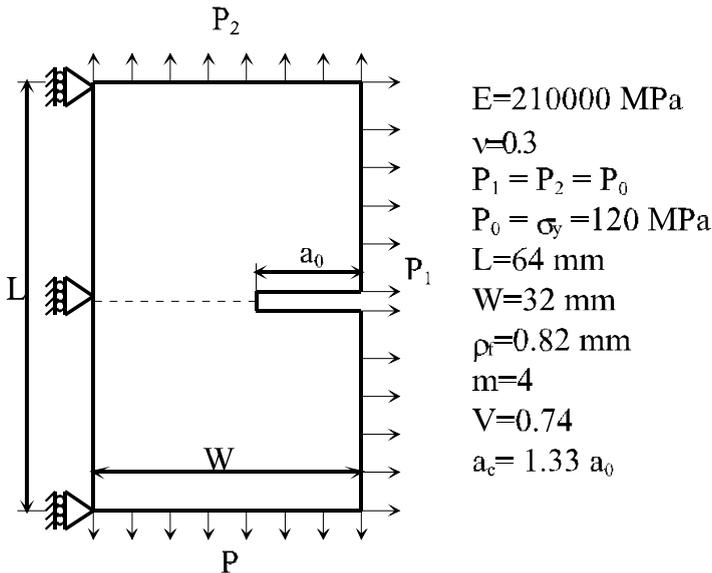


Figure 2: Rectangular plate with lateral crack.

The obtained results (fig. 3) show that the safety load is reduced in the presence of crack; this reduction depends on the crack length. The load P_2 amends adversely the domain of load and reduces the limits of the load obtained. While under the load P_1 , the influence of the crack is lesser because the stress concentration is less important, and is situated on crack lips, not in front of the crack tip.

Figure 4 shows well that in the case of a cracked plate, the dimensioning based on the limit analysis is not recommended if the applied loads could vary in an independent way.

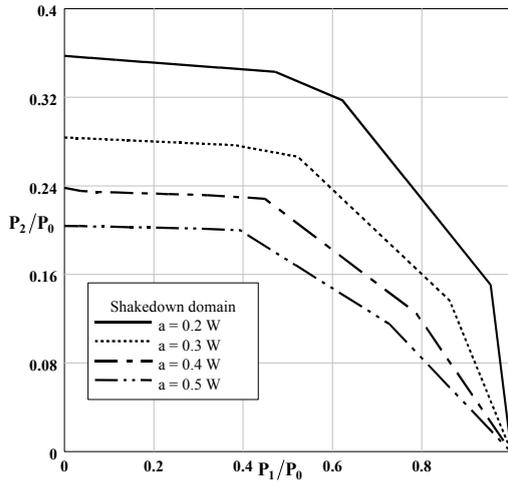


Figure 3: Shakedown domain.

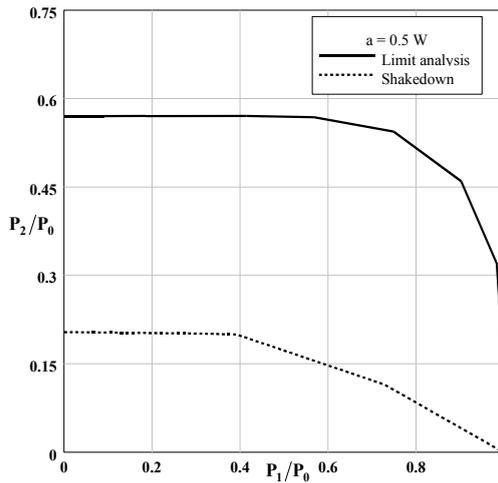


Figure 4: Shakedown and load limits domains.



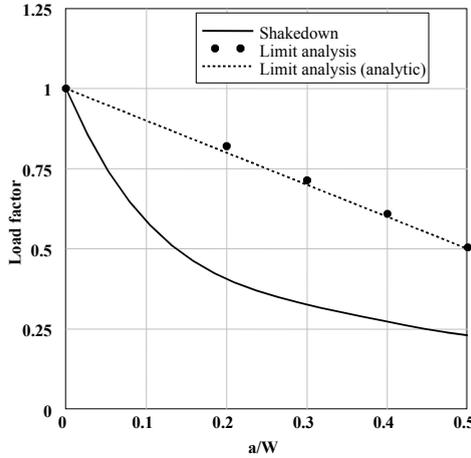


Figure 5: Influence of the ratio a/W on the load factor.

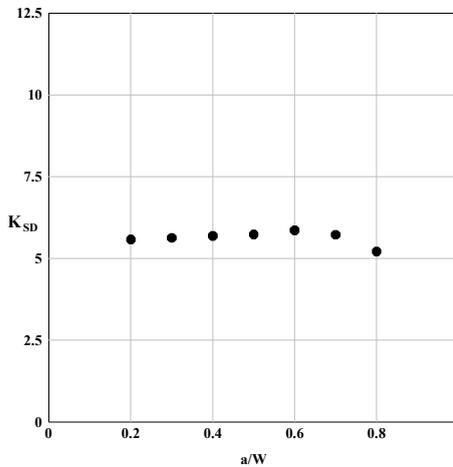


Figure 6: Independence of K_{SD} with a/W .

Figure 5 shows that for the cracked plate subjected to the load P_2 , the value of the limit load values agrees with the analytical ones.

6.2 Shakedown stress intensity factor K_{SD}

We have computed the stress intensity factor at the shakedown state for different crack lengths, the obtained results (fig 6), show that this factor K_{SD} is crack length independent, and it can be considered as the safety parameter against the cracked structure ruin, by plasticity and crack propagation for the mode I of loading.



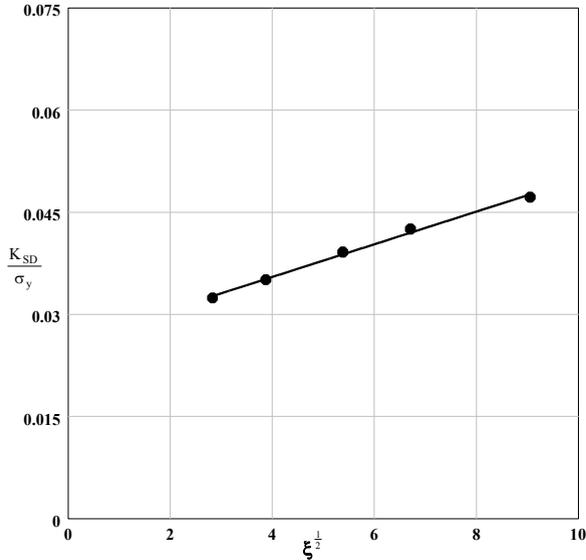


Figure 7: Relation between K_{SD} and $\xi^{1/2}$.

We have then studied the influence of the grain size on the shakedown stress intensity factor K_{SD} , figure 7 shows that the ratio K_{SD} / σ_y varies linearly with $\xi^{1/2}$.

$$\frac{K_{SD}}{\sigma_y} = a + b\xi^{1/2}. \quad (18)$$

By comparing this equation with the equation giving the fatigue threshold, one notes that the yield stress influences the shakedown stress intensity factor. This can be explained by the fact that the shakedown intensity factor is computed by taking into account the yield stress and the largest admissible crack length.

6.3 Comparison of K_{SD} and the fatigue threshold values for some materials

To validate the proposed approach, a comparison is made between the values of K_{SD} computed in the case of a rectangular plate solicted in mode I, the fatigue threshold K_S given by [7] and the shakedown stress intensity factor K_{sh} given by [5] for some materials. The characteristics of these materials are given in the Table 1.

According to the results given in the Table 2, one remarks that the values of K_{SD} agree with the values of K_{sh} given by [5]. On the other hand, one notices a disparity with the fatigue threshold K_S given by [7] for materials A and E. However, the results indicate that indeed K_{SD} can be considered as fatigue threshold.



Table 1: Diameter of the grain and mechanical material data.

Material	Designation	ξ (μm)	σ_y (MPa)
Docol 350	A	8	260
SS 141147	B	15	185
HP steel	C	29	210
HP steel	D	45	160
HP steel	E	82	120

Table 2: Fatigue threshold and shakedown stress intensity factors.

Material	K_S	K_{sh}	K_{SD}
A	5.4	8.9	8.43
B	6.0	7.6	6.49
C	6.2	9.5	8.22
D	6.7	8.1	6.8
E	8.2	7.8	5.73

More, knowing that the relationship existing between the yield stress and the diameter of the grain has the following form [7,8]:

$$\sigma_y = c + d\xi^{-\frac{1}{2}}, \quad (19)$$

σ_y increases when the diameter of the grain decreases and since the residual stress intensity increases with the increase of the elastic limit, one can say, considering the equation (16), that disparities observed between K_{SD} and K_S values are mainly caused by the residual stresses.

One concludes that the computed stress intensity factor can be considered as a fatigue threshold taking into account the residual stresses.

7 Conclusion

In this work, we have presented a new method in the prediction of the inelastic cracked structures failure by the application of the shakedown analysis. This new method presents several advantages, among which, the facility of computation since one undertakes only an elastic calculation. Also, it allows going more far than the elastic limit in the structures design. We can also compute the fatigue threshold using this method.

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